

APPENDIX IV

Seismic Behaviour of Current and Proposed CANDU 6 Reactors

IV-1. CANDU 6 WEIGHTS AND AREAS

IV-1.1 CANDU 6 Areas: See Figures 4-12 & 4-13

Pressure-bearing Area of ES (less area of cal tubes)

$$A_{ES} = (\pi/4)(300)^2 - 380((\pi/4)5.5^2) = 70\,685 - 9\,028 = \underline{61\,650 \text{ in}^2}$$

Cross-section area of Calandria tubes

$$A_{CT} = 380(\pi)5.0(0.05) = \underline{298 \text{ in}^2}$$

Cross-Section of Calandria Shell:

$$A_{CS} = 300(\pi)1.12 = \underline{1\,150 \text{ in}^2}$$

IV-1.2 CANDU 6 Weights:

W one end shield steel = both tube sheets + shell + 380[lattice tubes - LT holes]

$$= 0.283\{(\pi/4)(266)^2(5) + \pi(266)(1.5)36 + 380[\pi 5.5(0.4)36 - \pi/4((5.5)^2)5]\}$$

$$= 0.283\{278\,700 + 45\,200 + 49\,410\} = 0.283\{373\,300\} = 105\,650 \text{ lbs}$$

W cal shell = perimeter x length x t (ignore notch dia)

$$= 0.283\{\pi(300)(240)(1.12)\} = 0.283\{254\,500\} = 72\,020 \text{ lbs}$$

W one support shell & plate

$$= 0.283\{\pi(270)(1)36 + \pi(285)18(2.75)\} = 0.283\{74\ 860\} = 21\ 180\ \text{lbs}$$

W 380 cal tubes = perimeter x length x t

$$= 0.236\{\pi 5.0(0.05)(240)380\} = 0.236\{71\ 628\} = 16\ 900\ \text{lbs}$$

W one ES balls & water = (fraction balls + water){ gross vol ES - vol ES steel - lattice tube holes}

$$= \{0.65(0.283) + 0.35(0.036)\} \{(\pi/4)(266)^2(41) - 373\ 300 - (\pi/4)5.5^2(36)380\}$$

$$= \{0.197\} \{2\ 285\ 300 - 373\ 300 - 325\ 010\} = 0.197(1\ 586\ 990) = 312\ 640\ \text{lbs}$$

W h.water in cal = gross vol cal - vol cal tubes

$$= 0.039\{(\pi/4)(300)^2(240) - 380(\pi/4)5.1^2(240)\} = 0.039\{16\ 964\ 600 - 1\ 863\ 050\}$$

$$= 0.039\{15\ 101\ 550\} = 558\ 960\ \text{lbs}$$

W 380 full FCs = PT + (%vol x water) + fuel + EFs + Shield Plugs (Actuals via D. Brown)

$$= 380\{0.236(\pi 4.2(0.2)(240) + 0.6(0.039)(\pi/4)(5)^2(552) + 12(50) + [500 + 160]\}$$

$$= 380\{150 + 253 + 600 + [500 + 160]\} = 380 \times 1\ 663 = 631\ 940\ \text{lbs}$$

W RCUs = 32 SO/CA + 21 ADJ + 6 ZC + 6 LIS + 34 FD + 99 loc'r + 99 [thim/4 + GTX/4]

$$= 0.236[(\pi)(300)]\{ (32)0.6(5)(0.06) + 21(0.6)3.5(0.05) + 6(4.5)0.1 + 34(0.8)0.1\}$$

$$+ 99(20) + 0.283(99)\{ \pi(6)0.4(60) = 2\ 980 + 1\ 980 + 2\ 110 = 7\ 070\ \text{lbs}$$

W one Fuelling machine = 25 000 lbs

IV-1.3 CANDU 6 Total Weights

$$W_1 = \text{TOTAL DRY SHIPPED ASS'Y (excl FCs \& RCU)} = \underline{342\ 580\ \text{lbs}}$$

$$W_T = \text{GROSS TOTAL FULL REACTOR ASS'Y (excl. only FMs)} = \underline{2\,165\,830 \text{ lbs}}$$

IV-2. THERMAL EXPANSIONS

IV-2.1 Present Design: Unrestrained Thermal Expansions

Expansions for half-length ie. from mid-plane for symmetrical reactor: $\delta = L \times \alpha \times \Delta T$;

- (i) Calandria tube (install at 65°F [18°C]; operate at 150°F [65°C])

$$L_{\text{axial}} = 120''; \quad \alpha = 3.2(10^{-6}) / ^\circ\text{F}; \quad \Delta T = (150 - 65) = 85^\circ\text{F}$$

$$\delta_{\text{CT}} = 120 \times 3.2(10^{-6}) \times 85 = \underline{0.0326 \text{ ins}}$$

- (ii) Calandria shell (ignoring the annular plate) (install at 65°F ; operate at 150°F)

$$L_{\text{axial}} = 120'' \quad \alpha = 9.0(10^{-6}) / ^\circ\text{F} \quad \Delta T = (150 - 65) = 85^\circ\text{F}$$

$$\delta_{\text{CS}} = 120 \times 9.0(10^{-6}) \times 85 = \underline{0.0918 \text{ ins}}$$

- (iii) Concrete vault - axially (install calandria at 65°F; operate vault at 90°F [32°C] in mid-thickness)

$$L_{\text{axial}} = 120'' \quad \alpha = 3 \text{ to } 5 \times (10^{-6}) / ^\circ\text{F} \quad \Delta T = (90 - 65) = 25^\circ\text{F}$$

$$\delta_{\text{W}} = 120 \times 4.0(10^{-6}) \times 25 = \underline{0.0120 \text{ ins}}$$

IV-2.2 Traditional Design plus Bolted Support Plates Both Ends: See Figure 4-18.

Loads Under Restrained Thermal Expansion and Normal Pressure Load

- (i) Thermal Restraint Loads for Calandria with Sub-shells and Bolted Support Plates Both Ends

If they were free to expand, the calandria tubes and vault would expand by amounts shown in section IV-2.1, above.

However, in this structure, the calandria expands more than the vault, therefore the calandria tubes are compressed, and

the support plate at the vault end walls is compressed, to attain the same net deflection z .

Therefore, the calandria tube is compressed an amount:

$$\epsilon_{CT} = \delta_{CT} - z ; \quad \text{but } \epsilon_{CT} = P/k_{CT} \text{ under load } P \quad \text{(a) \& (b)}$$

and the support plates are dished outwards under the same force P , by the amount:

$$\epsilon_{SP} = z - \delta_w ; \quad \text{but } \epsilon_{SP} = P/k_{SP} \quad \text{(c) \& (d)}$$

Stiffness

$$A = 298 \text{ in}^2,$$

$$k_{CT} = EA/L \text{ for CT ;}$$

$$\text{and } E = 13(10^6),$$

$$L = 120 \text{ in,}$$

$$\text{then } k_{CT} = 13(10^6) \times 298/120 =$$

$$32.28(10^6) \text{ lb/in}$$

Stiffness $k_{SP} =$

$$25(10^6)$$

(Provided from FEM analysis for standard CANDU 6)

Equationa (a), (b), (c) & (d) are solved for P :

$$0.0326 - P/32.28(10^6) = P/25(10^6) - 0.0120$$

from which

$$P =$$

$$\underline{628\,355 \text{ lb}}$$

and $z = 0.0326 - 628\,355/32.28(10^6) =$

$$\underline{0.0131 \text{ ins}}$$

and Basic Compressive Stress in CT =

$$\sigma_{CT1} = \underline{628\,355/298 =}$$

$$\underline{2\,110 \text{ psi}}$$

(ii) Added Thermal Restraint Superimposed due to End Shield Dishing

With the same temperatures and distributions on tube sheets as in standard CANDU 6, the same self-balancing system of stresses is superimposed on the CTs, due to the central region of the end shields bowing apart from each other, balanced by the edge regions being pushed towards each other. The central CTs exhibit tensile stresses in the order of 3 000 psi, while the CTs near the edge exhibit stresses in the order of 4 500 psi, See Figure 2-27, ie,

Superimposed compressive stress on CTs near the edge =

$$\sigma_{CT2} =$$

$$\underline{4\,500 \text{ psi}}$$

(iii) Internal Pressure Loads

Use normal pressure at calandria mid-height is 4 psi cover gas pressure plus 12.5 feet static head:

Total force due to pressure acting on end shield net area = $(4 + 12.5(62.4/144))61\ 650 = 580\ 540$ lb

Tensile stress on CT = $580\ 540 / 298 =$ $\sigma_{CT3} =$ 1 950 psi

Then, total compressive stress on CT = $\sigma_{CT} = 2\ 110 + 4\ 500 - 1\ 950 =$ 4 660 psi

IV-2.3 Straight Main Shell Design with Rigid Support Both Ends:

Loads Under Restrained Thermal Expansion and Normal Pressure Load

(i) Thermal Restraint Loads for Straight Main Shell Calandria with Rigid Supports Both Ends

If they were free to expand, the calandria shell, calandria tubes and vault would expand by amounts shown in section IV-2.1, above.

However, in this structure, the calandria shell expands more than either the calandria tubes or the vault, therefore the calandria tubes are stretched, the calandria shell is compressed and the supports at the vault end walls are deformed outwards, to attain the same net deflection z .

Therefore, the calandria tube is stretched an amount:

$$\epsilon_{CT} = z - \delta_{CT}; \quad \text{but } \epsilon_{CT} = P_{CT}/k_{CT} \text{ under tension } P_{CT} \quad (\text{a) \& (b)}$$

and the supports are displaced outwards under the force P_w by the amount:

$$\epsilon_s = z - \delta_w; \quad \text{but } \epsilon_s = P_w/k_s \quad (\text{c) \& (d)}$$

and the calandria shell is compressed by the force P_s by amounts:

$$\epsilon_{CS} = \delta_{CS} - z; \quad \text{but } \epsilon_{CS} = P_{CS}/k_{CS} \quad (\text{e) \& (f)}$$

$$\text{also, } P_s + P_{CT} = P_{CS} \quad (\text{g)}$$

$$\text{Stiffness of calandria shell: } E = 29.5(10^6), \quad A_{CS} = 300\pi(1.12)^2 = 1055 \text{ in}^2, \\ L = 120 \text{ in,}$$

$$\text{then } k_{CS} = EA/L \text{ for CS} = 29.5(10^6)1055 / 120 = 260(10^6) \text{ lb/in}$$

Equation (a), (b), (c), (d), (e), (f) & (g) are solved for P_{CT} :

$$P_{CT} = \underline{1\,514\,000\text{ lb}}$$

and Basic Tensile Stress in CT = $\sigma_{CT1} = 1\,514\,000 / 298 = \underline{5\,080\text{ psi}}$

(ii) Added Thermal Restraint Superimposed due to End Shield Dishing

As for above calculation, the central CTs exhibit tensile stresses in the order of 3 000 psi, while the CTs near the edge exhibit stresses in the order of 4 500 psi, ie.,

Superimposed compressive stresses on CTs near the edge = $\sigma_{CT2} = \underline{4\,500\text{ psi}}$

(iii) Internal Pressure Loads

The total force due to pressure is the same as for the previous calculation, but this force is carried mostly on the CS, and only partly on CTs. If the total force is shared in proportion to their stiffnesses, then the CTs carry 32.28/ 260 of the total;

$$P_{CT2} = 1\,514\,000 \times 32.28 / 260 = \underline{188\,000\text{ lb}}$$

and Tensile stress on CT = $\sigma_{CT3} = 188\,000 / 298 = \underline{630\text{ psi}}$

Therefore, total tensile stress on CT =

$$\sigma_{CT} = 5080 - 4\,500 + 630 = \underline{1\,210\text{ psi}}$$

(iv) Compressive Load in Calandria Shell

From equations in (i): $P_{CS} = \underline{3\,200\,000\text{ lb}}$

and compressive stress in Calandria Shell =

$$\sigma_{CS} = P_{CS} / A_{CS} = 3\,200\,000 / 1155 = \underline{3\,030\text{ psi}}$$

(v) Effect of Stiffer Support

It is noted that the preferred design will have the annular Support Shell welded directly to the embedment, rather than

using the present Bolted Support Plate. In this case, the stiffness of the support would more closely approach that of the vault wall itself, which is estimated to be $100 (10^6)$ to $150(10^6)$ lb/in. If the Support stiffness is taken as $k_s = 100(10^6)$ lb/in in the above solution, we find little change in the Calandria Tube load:

$$P_{CT} = 1\,274\,000 \text{ lb}$$

$$\text{and Basic Tensile stress} = \sigma_{CT_1} = 1\,274\,000 / 298 = \underline{4\,280 \text{ psi}}$$

Adding the Stresses due to the End Shield and due to Internal Pressure:

$$\text{Total tensile stress on CT} = \sigma_{CT} = \underline{4\,280 - 4\,500 + 630} = \underline{410 \text{ psi}}$$

$$\text{and Calandria Shell Load becomes: } P_{CS} = 7\,280\,530 \text{ lb}$$

$$\text{and compressive stress on CS is } \sigma_{PCS} = 7\,280\,530 / 1155 = \underline{6\,300 \text{ psi}}$$

Both of these stresses are still very acceptable.

IV-3.0 SEISMIC BEHAVIOUR

IV-3.1 Present Reactor Design

(i) Basic Modes: See Figures 4-14, -15 & -16.

The RSA system may be seen as the two End Shield masses, each with half of all the included masses* attached; the two halves are connected and supported through several paths, but principally the calandria tubes and the bolted support plate. The entire system is supported in the embedment rings in the vault walls. There are two principal modes of seismic response motion: the first mode is due to the mass of the *unbolted* half being driven by the spring of the calandria tubes; the second is due to the total mass of *both* halves being driven as a whole by the spring of the *support plate*.

$$\begin{aligned} \text{Masses } M/2 &= W_T / 2(386) = 2\,805 \text{ slugs} \\ M_f &= (W_{FM} + W_{FO}/2) / 386 = 67 \text{ slugs} \end{aligned}$$

Their seismic response motions occur at their resonant frequencies, found as:

$$f_1 = (1/2\pi)\sqrt{K_{ex}/(M/2 + M_d)} = (1/2\pi)\sqrt{16.14(10^6)/(2\ 872)} = \underline{11.7\ Hz}$$

$$\text{and } f_2 = (1/2\pi)\sqrt{K_{ex}/(M + M_d)} = (1/2\pi)\sqrt{25(10^6)/5\ 677} = \underline{10.6\ Hz}$$

(ii) End Shield Mode:

The inner region of *each* mass moves relative to the outer region. This mode has frequency:

$$f_3 = (1/2\pi)\sqrt{2K_{ex}/(2M/4 + M_f)} = (1/2\pi)\sqrt{2(2.5)(10^6)/(2\ 872)} = \underline{6.6\ Hz}$$

(iii) FM at non-PA End:

Here, the mass of the fuelling machine at the *non*-positioner end is driven by the spring of the pressure tube itself. Its frequency is:

$$f_4 = (1/2\pi)\sqrt{K_{pt}/M_f} = (1/2\pi)\sqrt{143\ 000/67} = \underline{7.4\ Hz}$$

Where Stiffness $K_{pt} = EA/L$ for PT: and $E = 13(10^6)$, $A = 4.2\pi(0.2)^2 = 2.64\ \text{in}^2$, $L = 240\ \text{in}$

$$\text{then } K_{pt} = \underline{143\ 000\ \text{lb/in}}$$

IV-3.3 Straight Mainshell design with Rigid Supports

(1) Basic Mode: See Figure 4-20.

The support plate stiffness is selected to raise the frequency of the fifth mode sufficiently to reduce PA and RJ loads, but without applying compression to calandria tubes. The calandria shell stiffness is higher than that of the vault walls, and its expansion will dominate this condition. The support is directly welded to the embedment, rather than bolting the support plate. As noted in thermal calculations above, the support stiffness will approach that of the vault end wall itself, and a value midway between the two is used, $60(10^6)$ lb/in.

The frequency found for f_5 is raised to be:

$$f'_5 = (1/2\pi)\sqrt{2K'_{bs}/(2M/2 + M_f)} = (1/2\pi)\sqrt{(2)60(10^6)/5\ 677} = \underline{23.1\ Hz}$$